

Wave Transformation, Prediction, and Analysis at Kaunalapau Harbor, Lanai, Hawaii

Jessica H. Podoski, P.E.

Coastal Engineer, USACE Honolulu District

Christopher Goody, P.E.

Sea Engineering, Inc.

Thomas D. Smith, P.E.

Coastal Engineer, USACE Honolulu District

**12th International Workshop on
Wave Hindcasting and Forecasting**

Waikoloa, Hawaii

November 3, 2011



US Army Corps of Engineers
BUILDING STRONG.



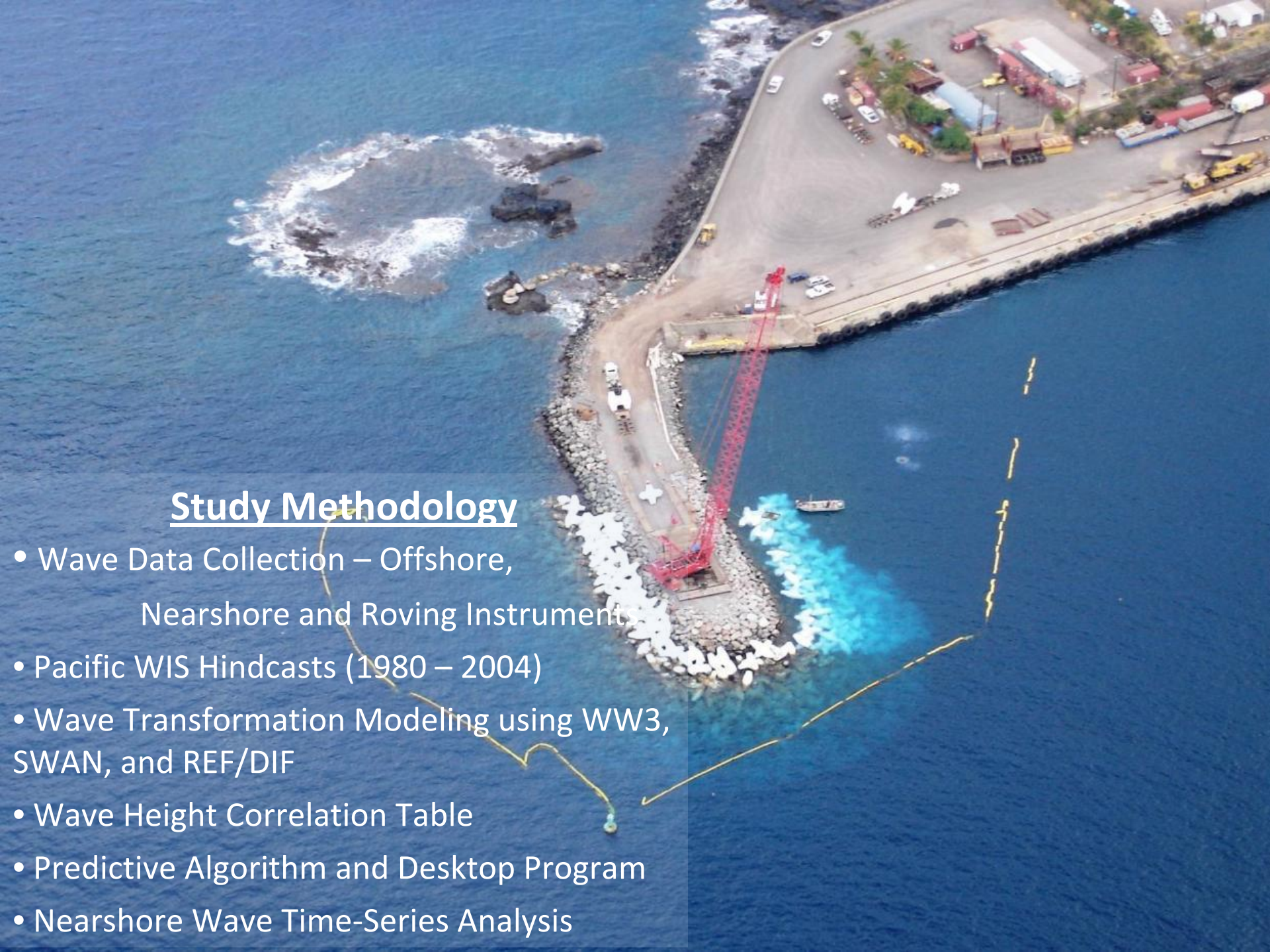


Study Motivation

Kaumalapau Harbor Breakwater Repair selected for monitoring under the USACE Monitoring of Completed Navigation Projects (MCNP) Program

Program Goals:

- 1) Measure waves at barge pier to determine repaired breakwater's effect on mooring conditions
- 2) Evaluate post-construction wave climate incident to breakwater for use in assessing structure stability and evaluating armor unit movement

An aerial photograph of a coastal area. In the upper right, there is a paved area with several buildings, including a large white one, and many parked vehicles. A long pier extends from this area into the water. On the pier, there is a prominent red crane. To the left of the pier, the water is dark blue, and there are some rocky outcrops with white foam from breaking waves. A yellow line, possibly a buoy line, stretches from the pier towards the bottom right of the frame. The text 'Study Methodology' is overlaid on the left side of the image.

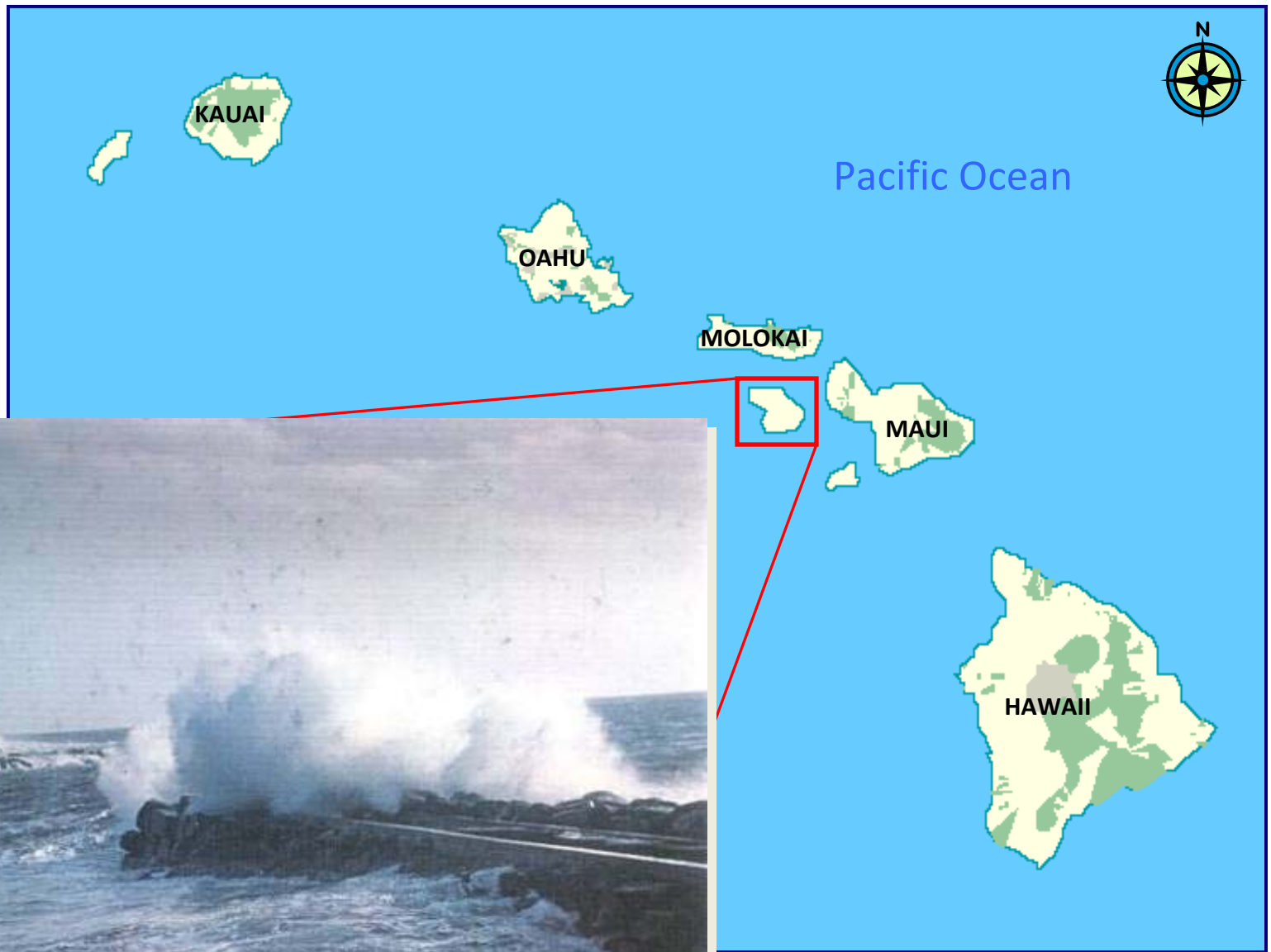
Study Methodology

- Wave Data Collection – Offshore, Nearshore and Roving Instruments
- Pacific WIS Hindcasts (1980 – 2004)
- Wave Transformation Modeling using WW3, SWAN, and REF/DIF
- Wave Height Correlation Table
- Predictive Algorithm and Desktop Program
- Nearshore Wave Time-Series Analysis

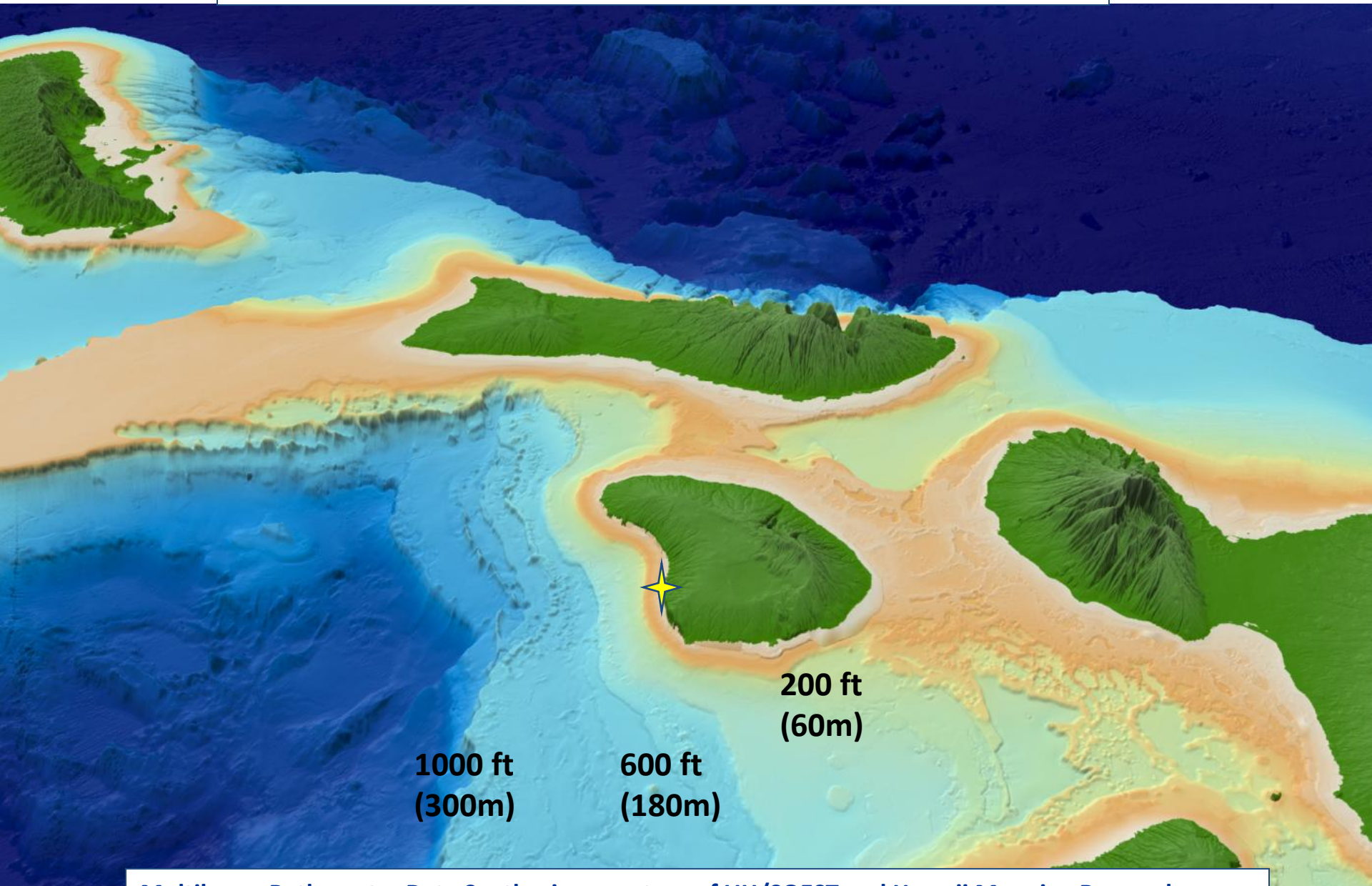


Summary of Conclusions

- Incident post-construction waves at repaired breakwater have been 10 ft or less so far
- These waves have not been large enough to affect breakwater armor layer stability
- Mooring conditions at barge pier are highly dependent on sea/swell direction and period
- Conditions at barge pier are significantly improved during the post-construction conditions observed



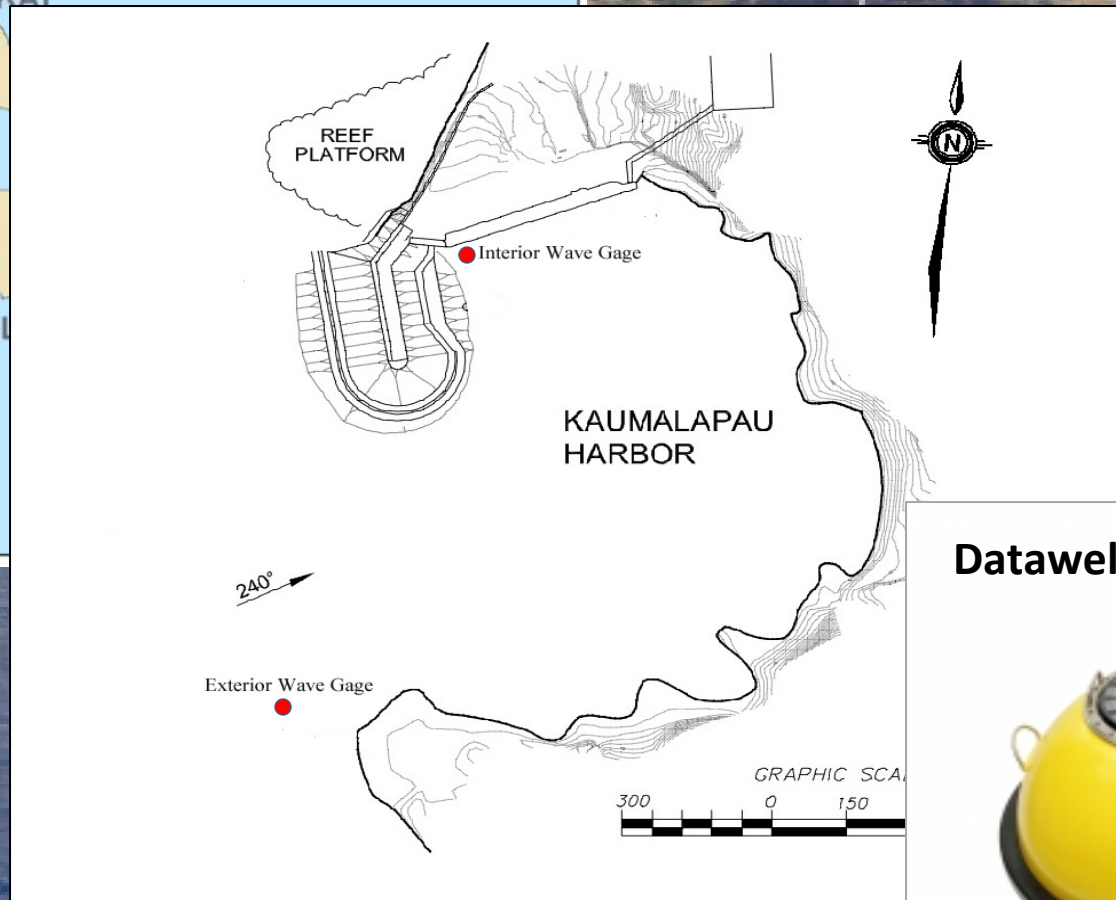
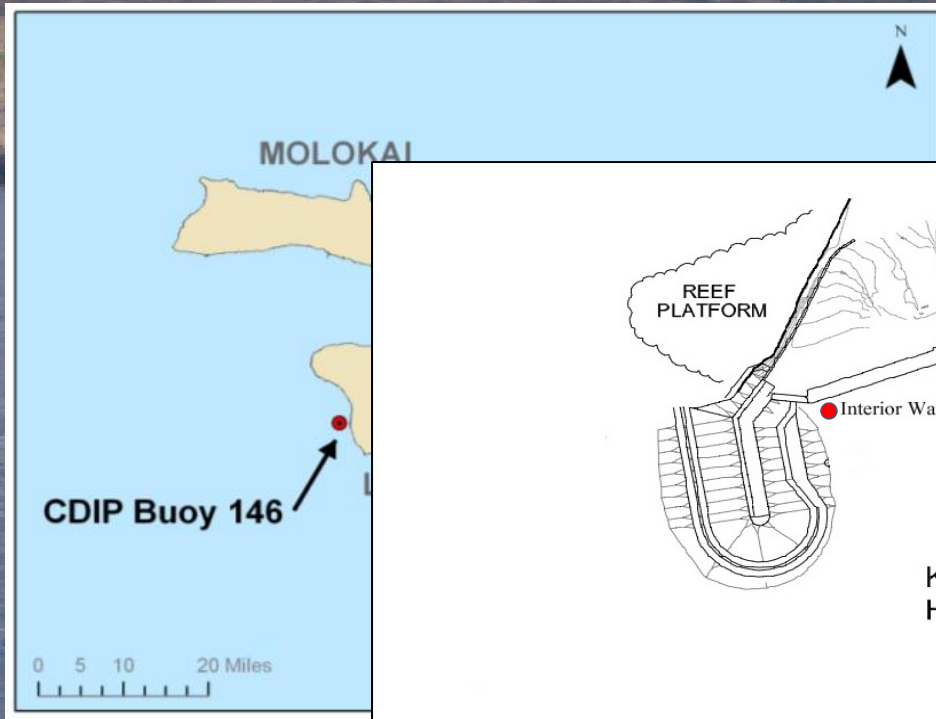
3D Bathymetry Near Kaumalapau Harbor



Harbor Layout



Wave Data Collection



Datawell Roving Buoy

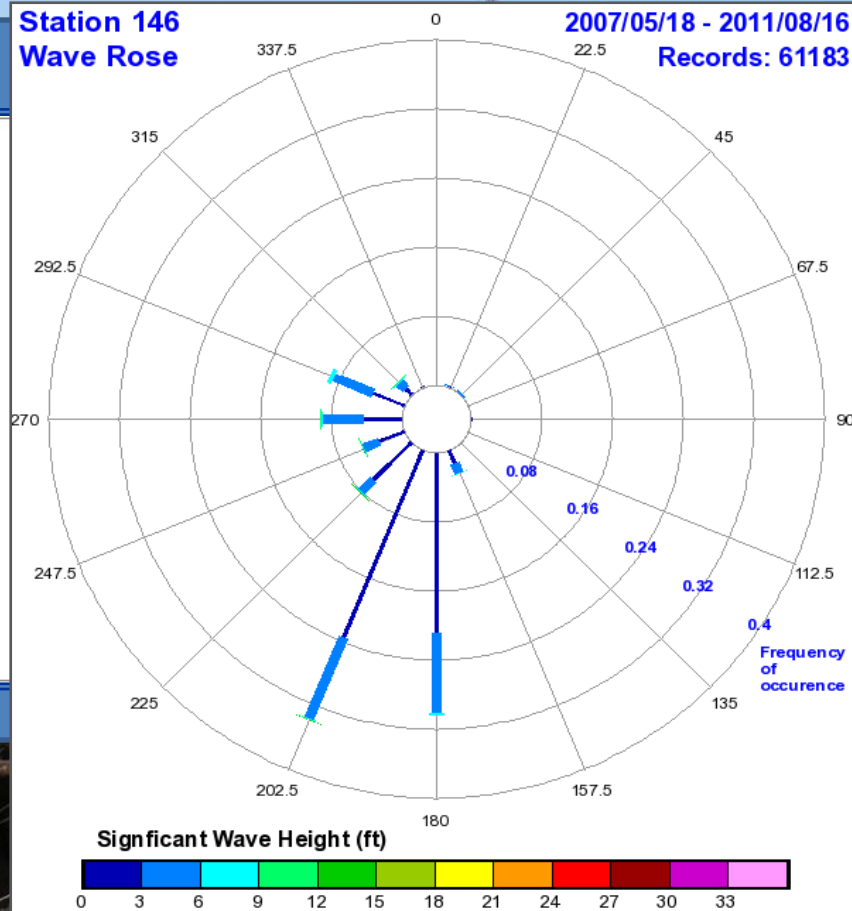


Wave Data Collection

Offshore CDIP Buoy

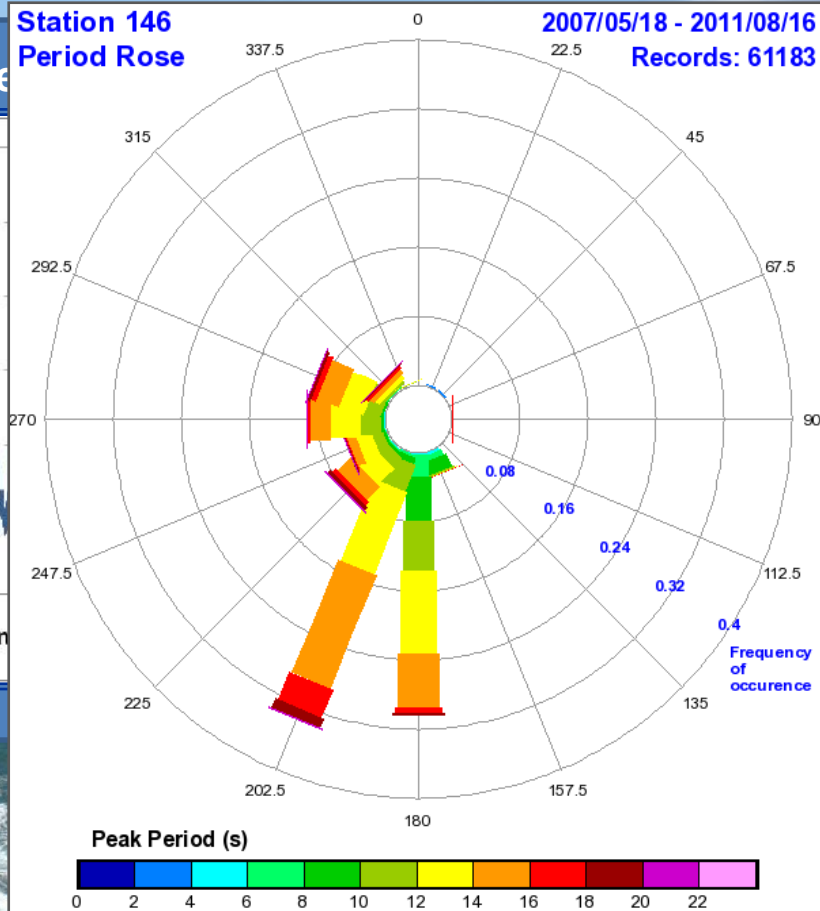
Station 146
Wave Rose

2007/05/18 - 2011/08/16
Records: 61183



Station 146
Period Rose

2007/05/18 - 2011/08/16
Records: 61183



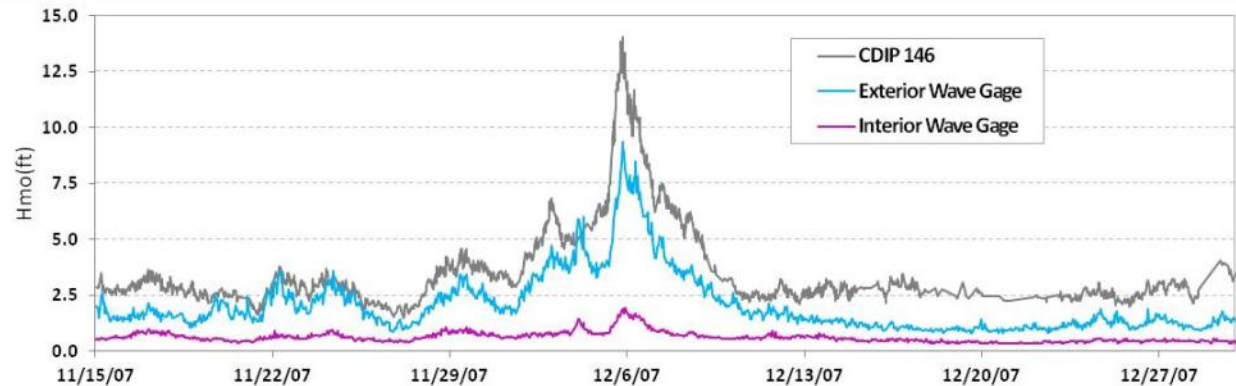
Wave Data Collection

Offshore CDIP Buoy and Harbor Gages

December 2007 Kona Storm:

$T_p = 8.3$ s

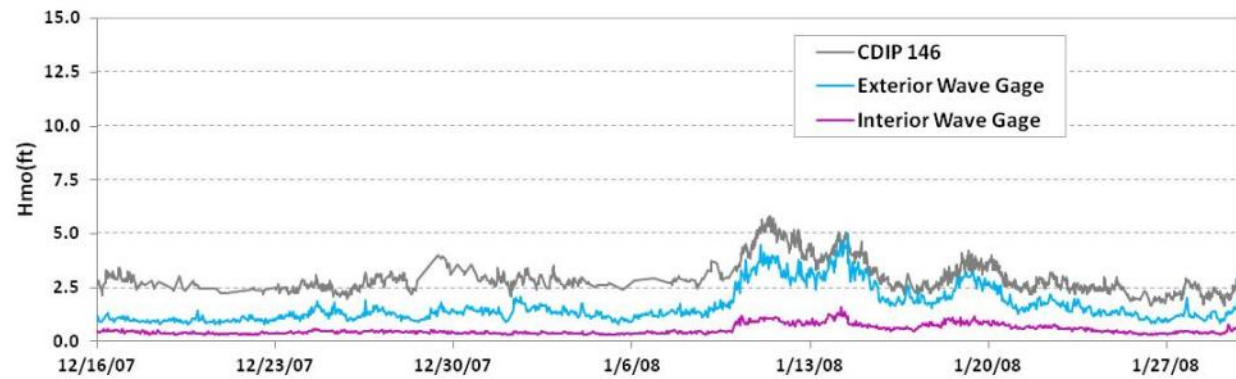
Dir = 224 TN (SW)



January 2008 W-NW Swells:

$T_p = 15.4$ s

Dir = 298 TN (NW) &
281 (W-NW)



November 2007 South Swell:

$T_p = 16.7$ s

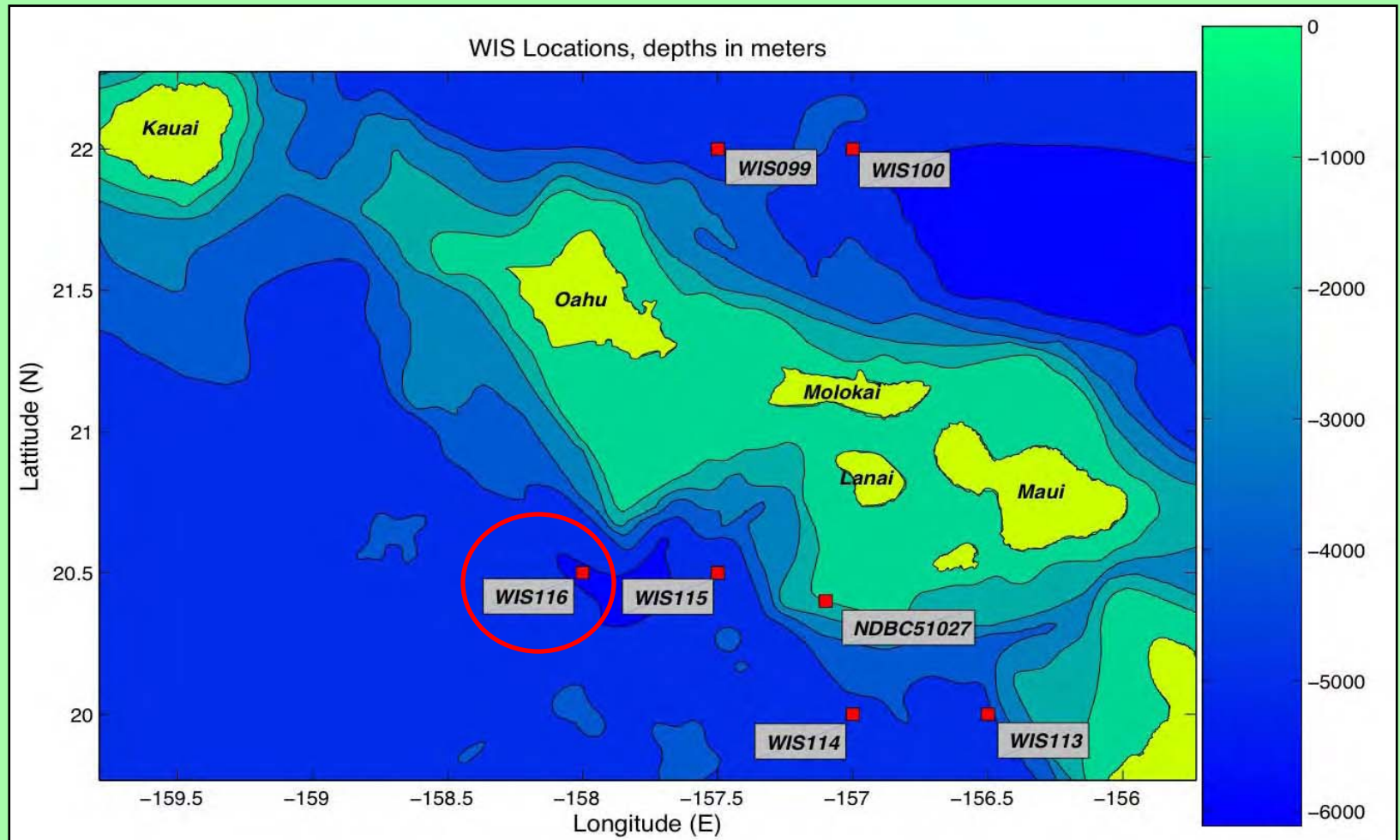
Dir = 190 TN (S-SW)



➤ **Relative Wave Response at the Barge Pier is Highly Dependent on Wave Period and Direction**

Wave Transformation

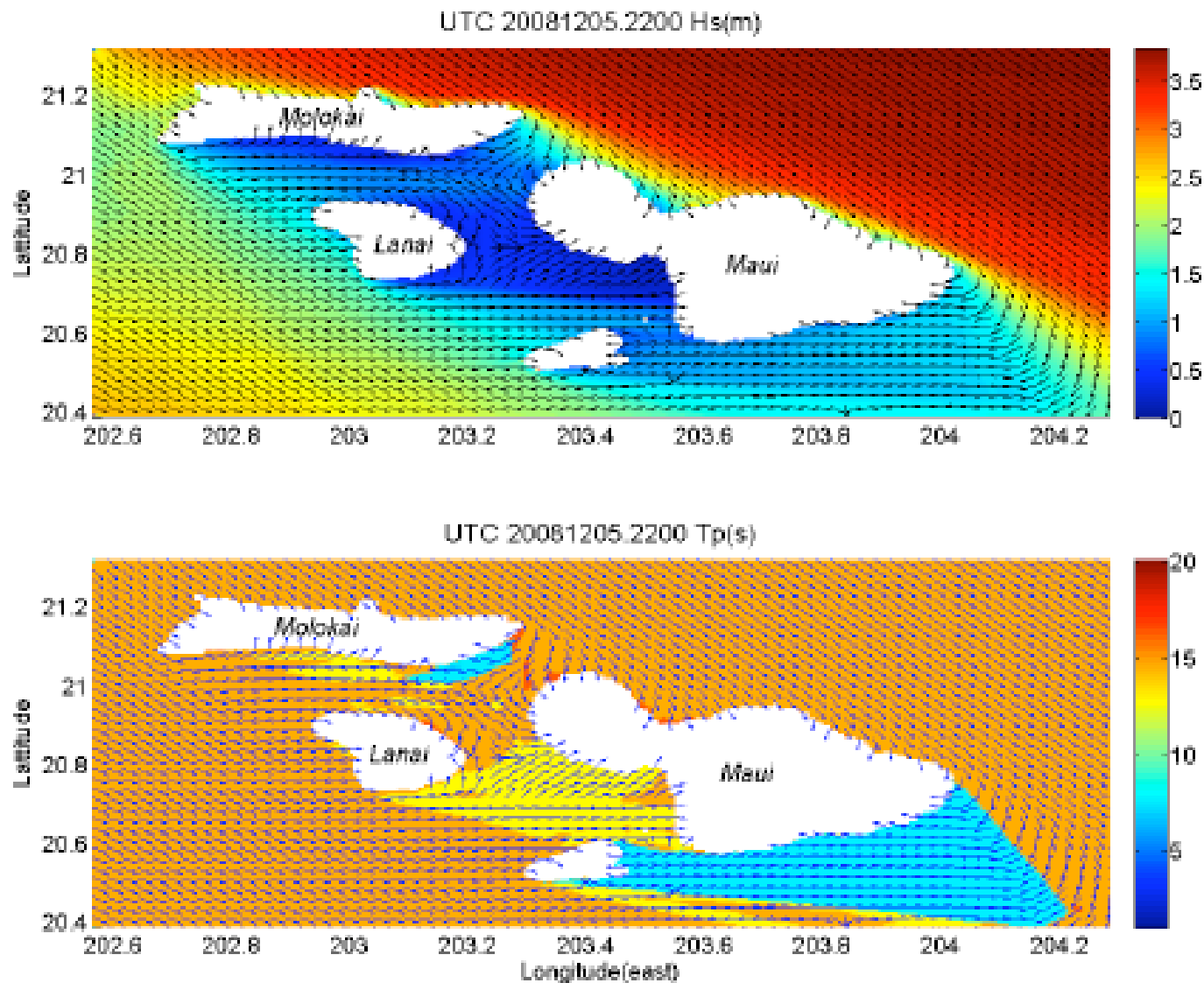
WIS Hindcast Data



- WIS Pacific Hindcast available for 1980 – 2004
- Stations 113, 114, 115, 116 compared to 1 year of buoy data at NDBC 51027
- WIS Station 116 had best fit overall for Tradewind, South Swell and NW Swell

Wave Transformation

SWAN Model



18 Aug 2007 – 28 October 2007

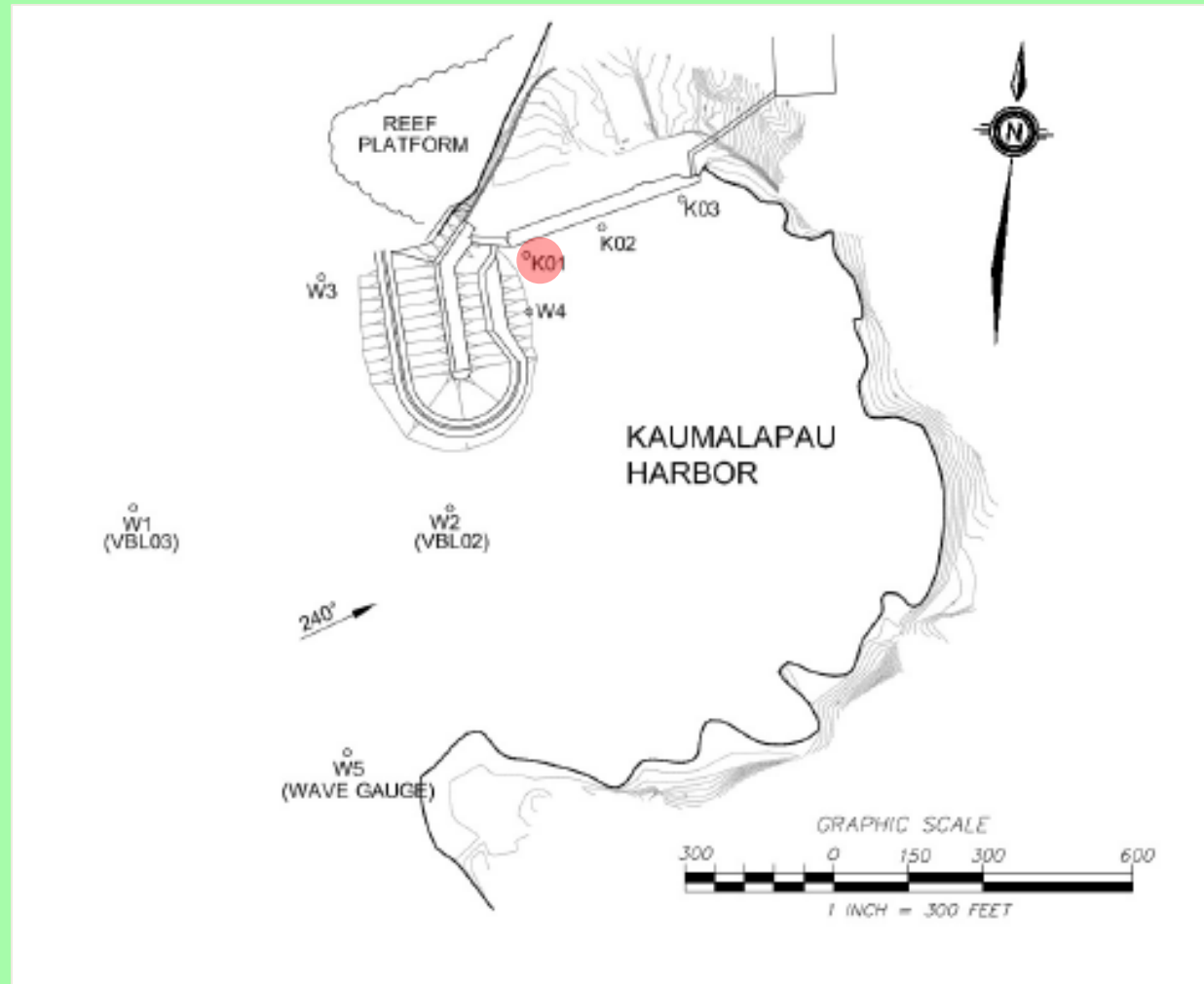
Wave Transformation

REF/DIF Model

CDIP 146 Buoy

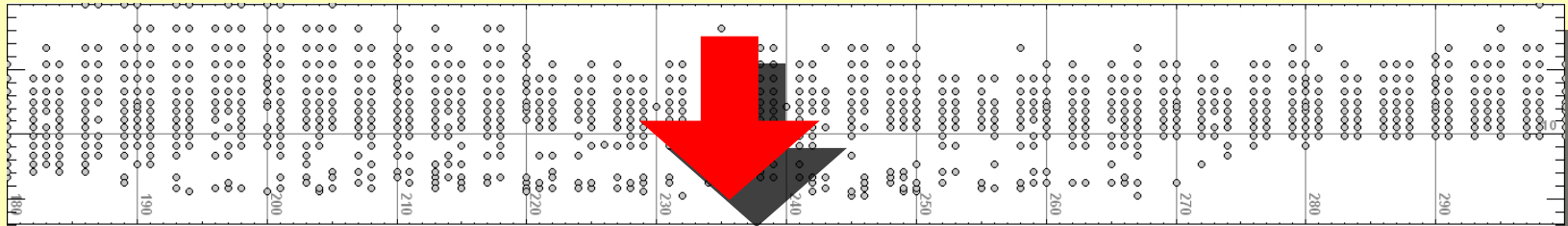


- REF/DIF used to transform SWAN output into harbor (tradewind cases dropped)
- Eight observation stations
- Model validation through comparison of A' with 3D physical model data (1998) => negative bias
- A' relationship at observation stations for 119 discrete wave conditions

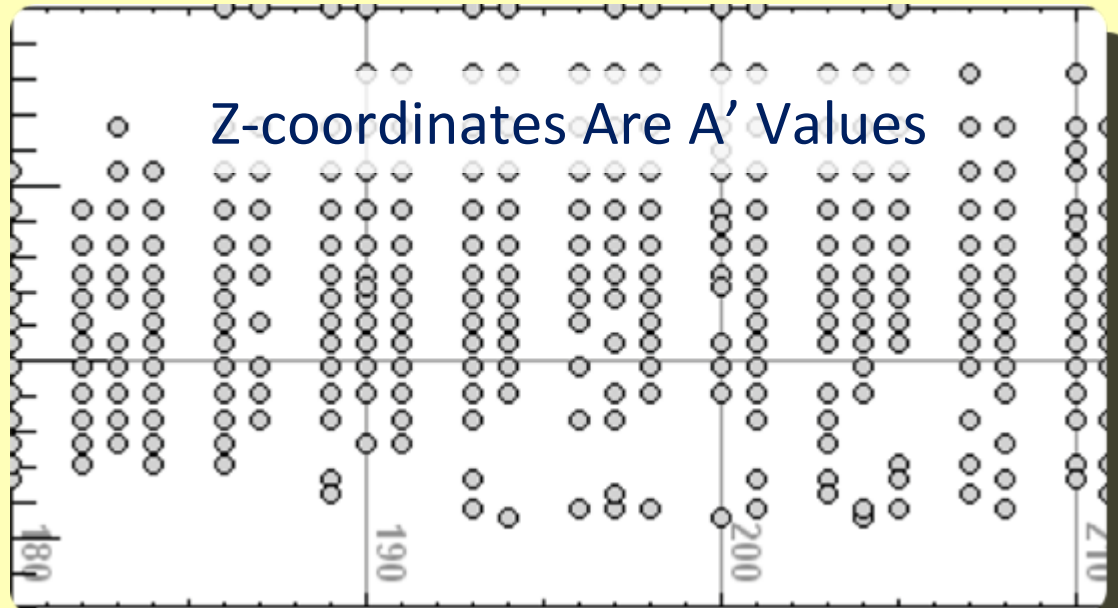


Wave Prediction

Surface Plots and KPWAVE



Y-coordinate
= Wave Period



X-coordinate = Wave Direction

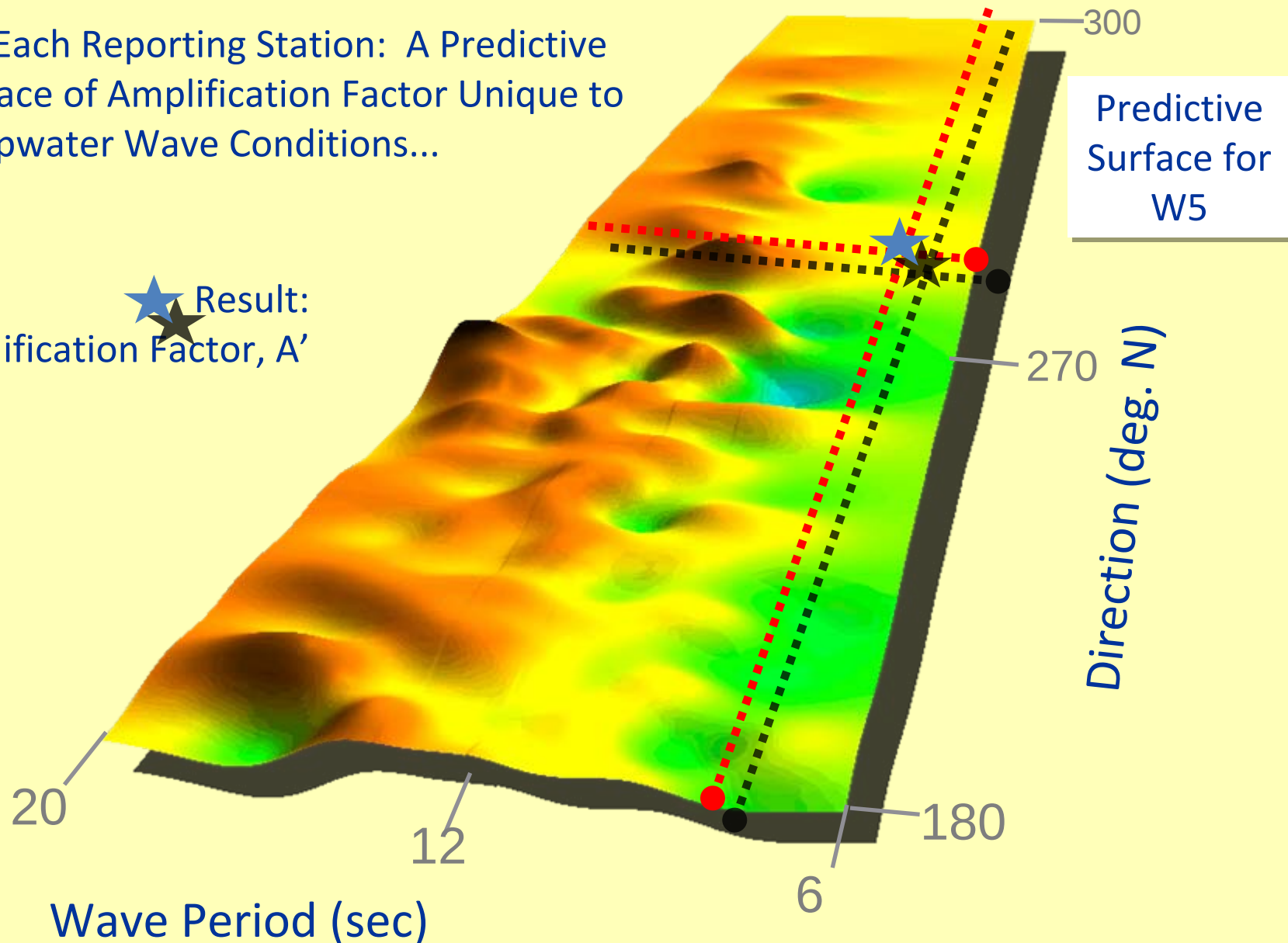
Time Series Data Sets Were Used to Calculate Amplification Factor for Each Measured Set of Conditions, Yielding a tabulation of 'x,y,z' values (direction, period, A' coefficient)

Wave Prediction

Surface Plots and KPWAVE

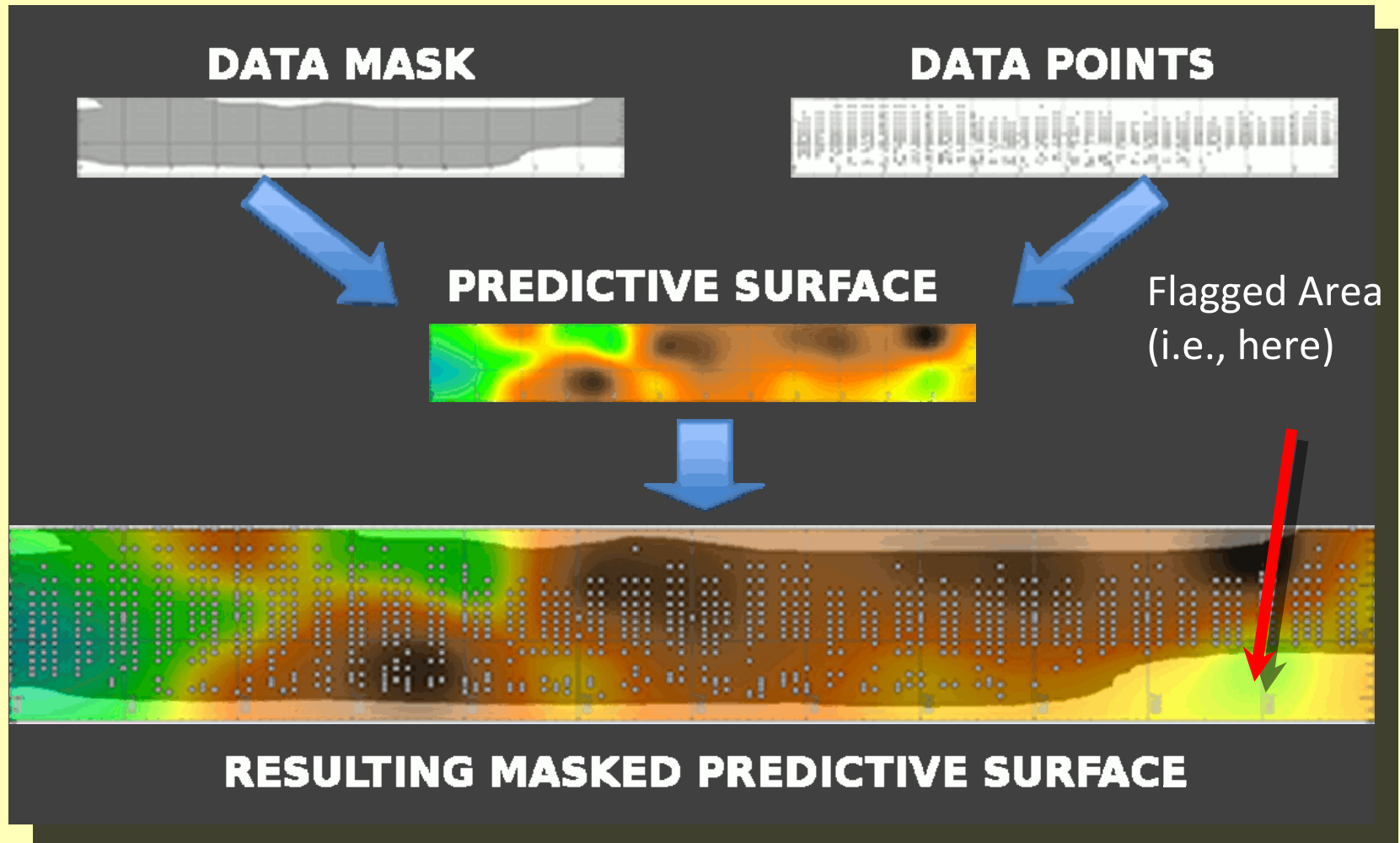
For Each Reporting Station: A Predictive Surface of Amplification Factor Unique to Deepwater Wave Conditions...

★ Result:
Amplification Factor, A'



Wave Prediction

Surface Plots and KPWAVE



The Predictive Surface is Filtered to Identify Values That Are Too Far From Real Data Points (Points of Low Confidence)

Wave Prediction

Surface Plots and KPWAVE

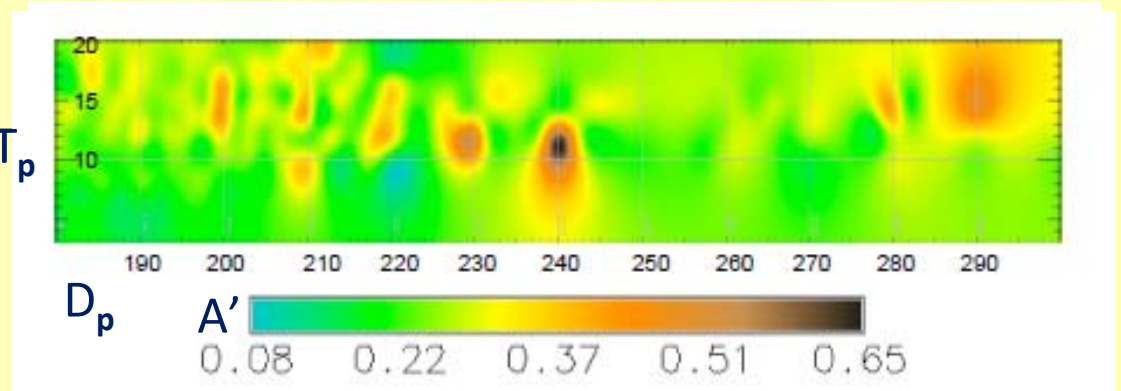
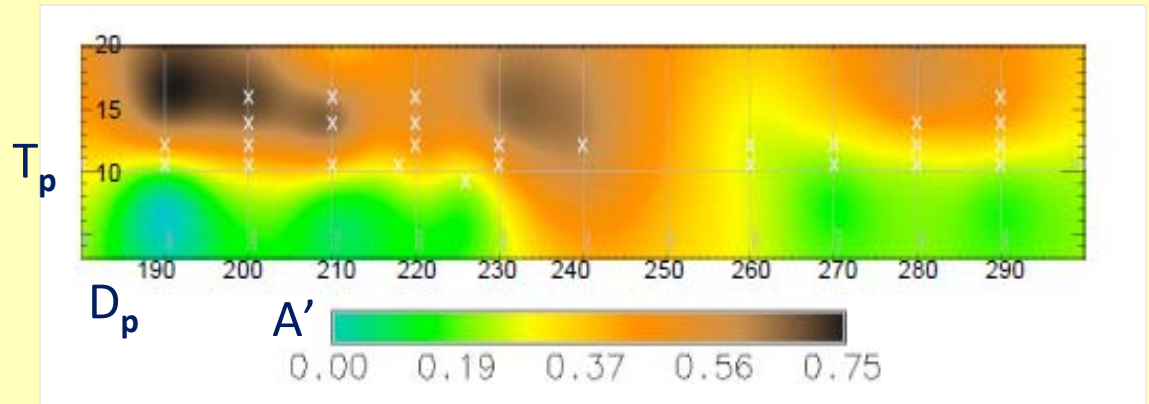
- Difference between *predicted* and *measured* A' at Location K01:

+0.49 (over predicted) for wave periods 15 - 20 seconds coming from S through SSW(180 – 215 TN)

-0.21 (under predicted) for shorter wave periods (5-10s) coming from the south (190 TN).

- Predictive Surfaces (based only on model output) created at K02, K03, W2, W3, W4, and W5 output wave stations also

3D Surface Plot of Amplification Factor at Location K01 (Model Data)



3D Surface Plot of Combined A' (Measured/Model Data)

Wave Prediction

Surface Plots and KPWAVE

- KPWAVE – a Fortran program that uses predictive algorithms to estimate wave amplification factor based on wave direction and period
- User can enter parameters manually or use time series input file from the CDIP buoy website (up to 1 year)
- Creates an output file of the corresponding time series at 7 output locations
- Flags for “Extrapolated” values or “Out of Bounds”

[illegible]

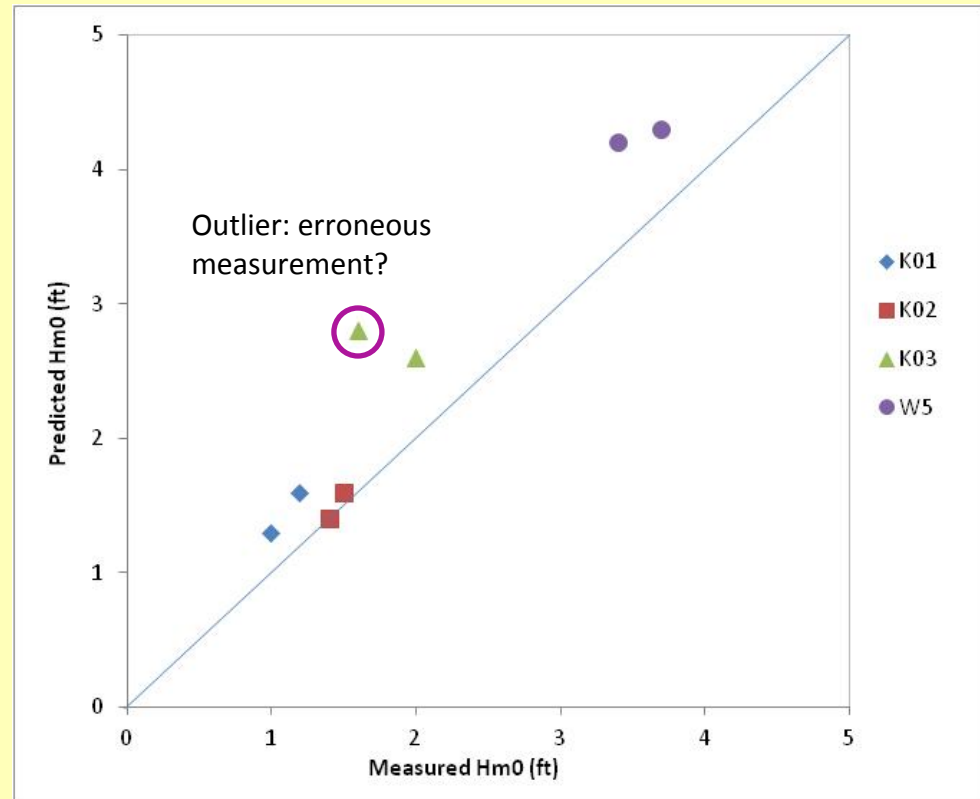
Wave Prediction

Surface Plots and KPWAVE

- Comparison of predicted H_s from modeling to roving buoy data on 5 Dec 08 at barge pier stations and harbor entrance
- Positive bias in modeled data
- One potential erroneous point
- Sensitivity to T_p and D_p



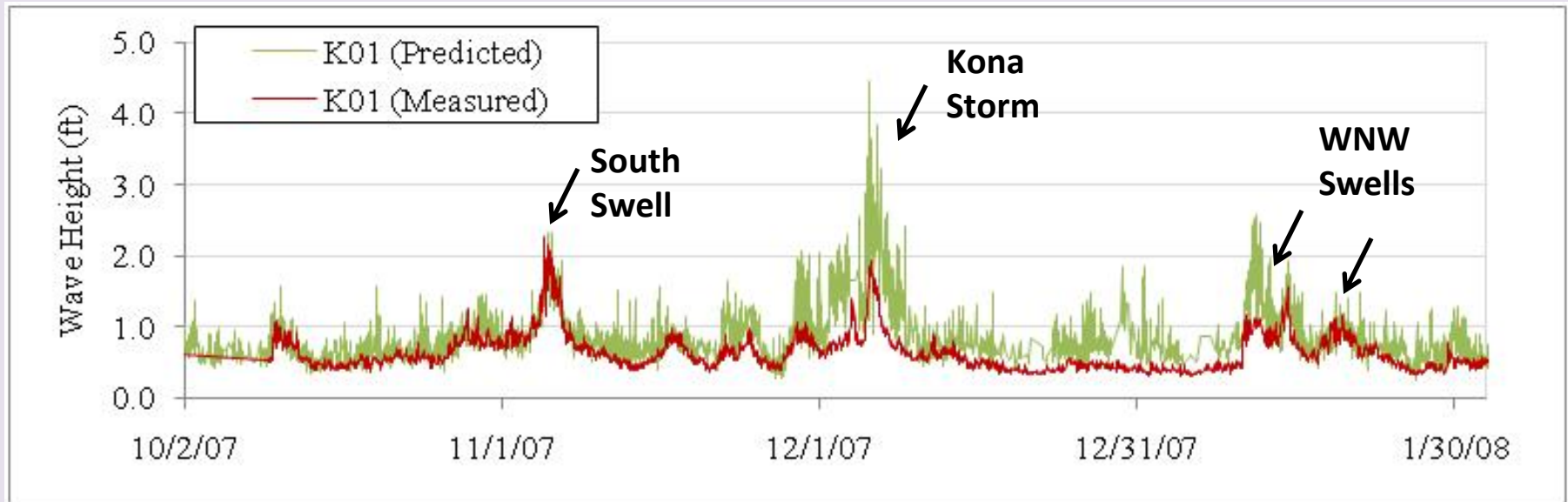
Comparison of Measured vs. Predicted H_s
for 5 Dec 2008 at Specified Locations



Moderate NW Swell Conditions:
 $H_o = 3.7 - 4.3$ ft, $T_p = 13.8 - 15.4$ s, Dir = 267-286 TN

Time-Series Analysis

KPWAVE vs. Gage Data at Barge Pier



Measured vs. Predicted Wave Height at K01

- Good agreement for South Swell and W-NW “Swell 2”

Possible reasons for overprediction during Kona storm and W-NW “Swell 1”:

- Kona/WNW wave conditions were not adequately represented in the model runs completed to create the lookup table (requiring extrapolation by KPWAVE)
- Limited wave gage measurements from K01 that were incorporated into this predictive surface for augmentation did not include these conditions
- REF/DIF not accurately representing the diffraction occurring in the lee of the breakwater under these conditions – further calibration required?

Time-Series Analysis

Predicted Waves along Barge Pier

Offshore Event of >6 feet : Kona, South, WNW or Hurricane

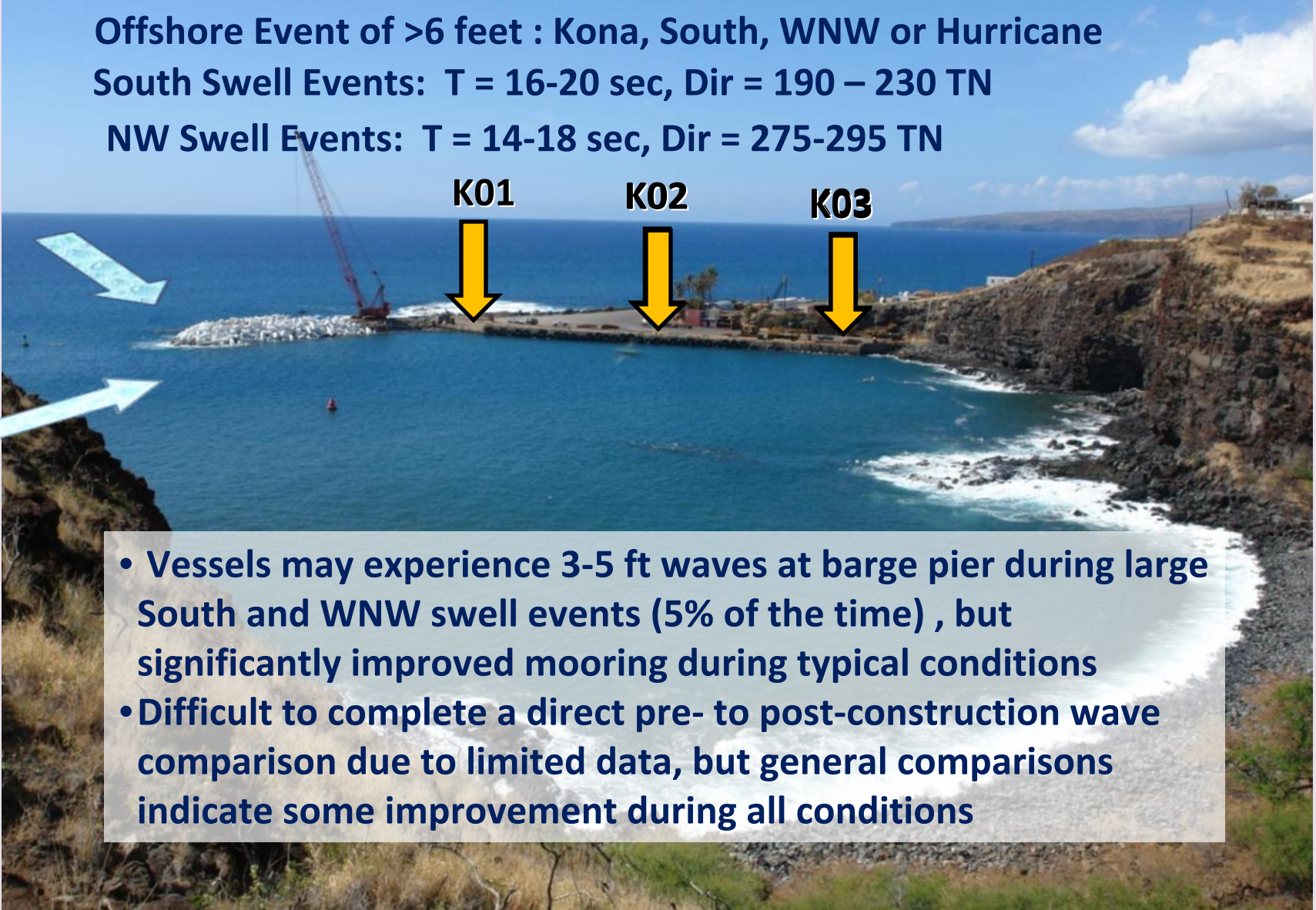
South Swell Events: $T = 16-20$ sec, $Dir = 190 - 230$ TN

NW Swell Events: $T = 14-18$ sec, $Dir = 275-295$ TN

K01

K02

K03

- 
- Vessels may experience 3-5 ft waves at barge pier during large South and WNW swell events (5% of the time) , but significantly improved mooring during typical conditions
 - Difficult to complete a direct pre- to post-construction wave comparison due to limited data, but general comparisons indicate some improvement during all conditions

Time-Series Analysis

Incident Wave at Breakwater



Conclusions

- Wave data collected in deep water and in lee of completed breakwater have documented wave climate inside/outside harbor
- Automated method to determine wave heights within harbor based on permanent CDIP Station and numerical modeling => KPWAVE (also beneficial to harbor users)
- Wave response and mooring conditions at barge pier are highly dependent on wave direction and wave period
- Long period waves (15 – 20s) from WNW and SSW, and Kona storms will still result in wave energy and surge in harbor
- The breakwater has improved mooring conditions within the harbor thus far, especially during moderate NW swells
- Nothing close to design wave has been experienced since completion of breakwater (largest recorded deep water $H_s = 14$ feet) => not large enough to affect breakwater armor layer stability

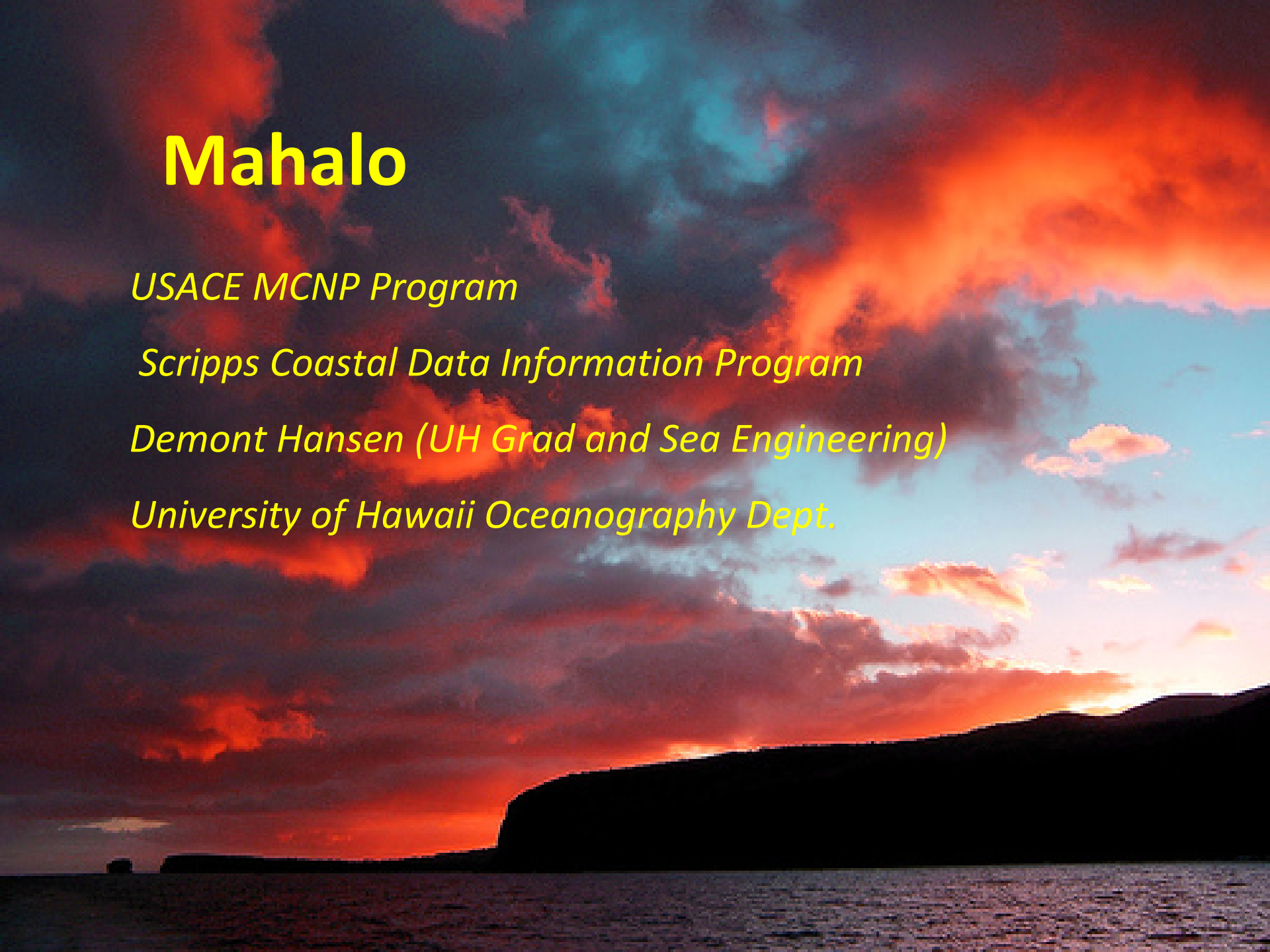
Mahalo

USACE MCNP Program

Scripps Coastal Data Information Program

Demont Hansen (UH Grad and Sea Engineering)

University of Hawaii Oceanography Dept.



Thank You

